

Interpretation of Pile Load Tests

Ken Gavin
Professor of Subsurface Engineering
TU Delft

21st October 2021

Background

Aim: Consider how pile tests can be used to derive national or international design methods with a focus on base resistance

- Validate design or optimise?
- Project or national application?
- Tests on comparable sites?



Dutch Practice NEN 9997-1

$$q_b = \alpha_p q_{c;Dutch}$$

- Koppejan 4D/8D averaging method
- Limiting base resistance = 15 MPa
- 2016 update: 30% reduction in α_p

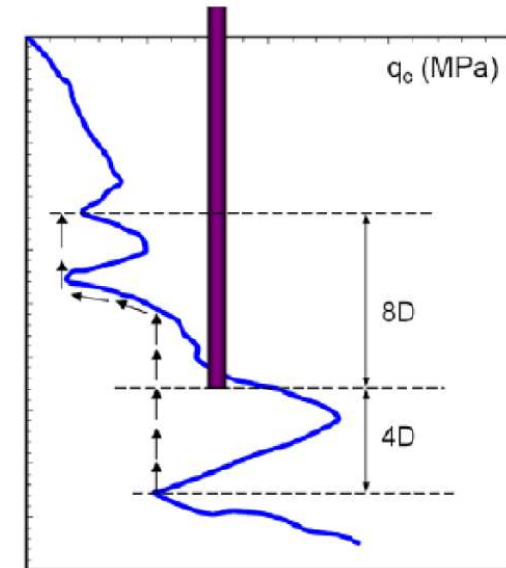


Table 1: Reduction factors for selected pile types from NEN 9997-1

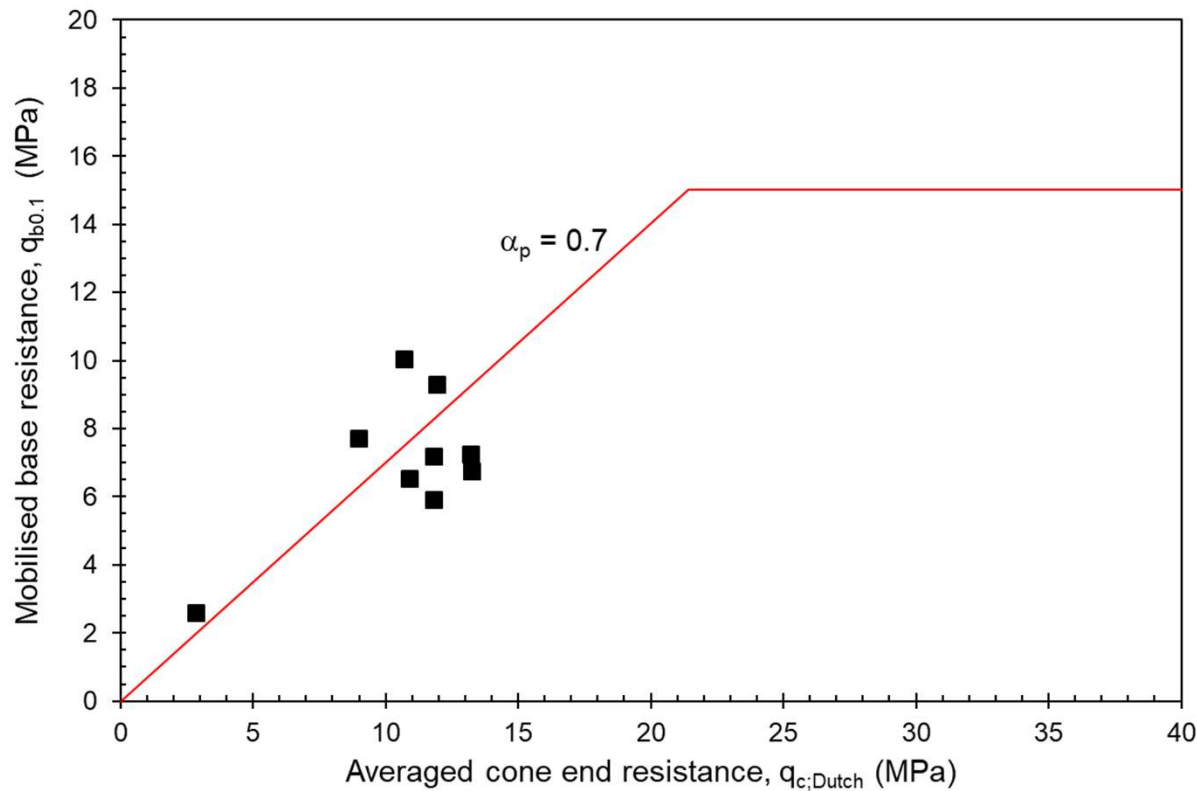
| Pile type | α_p | α_s |
|--------------------------|------------|------------|
| Bored pile | 0.35 | |
| Continuous flight auger | 0.56 | |
| Driven cast-in-situ | 0.70 | |
| Driven precast | 0.70 | |
| Screw injection | 0.63 | |
| Steel tubular (open-end) | 0.7 | |

Dutch Database of instrumented pile tests



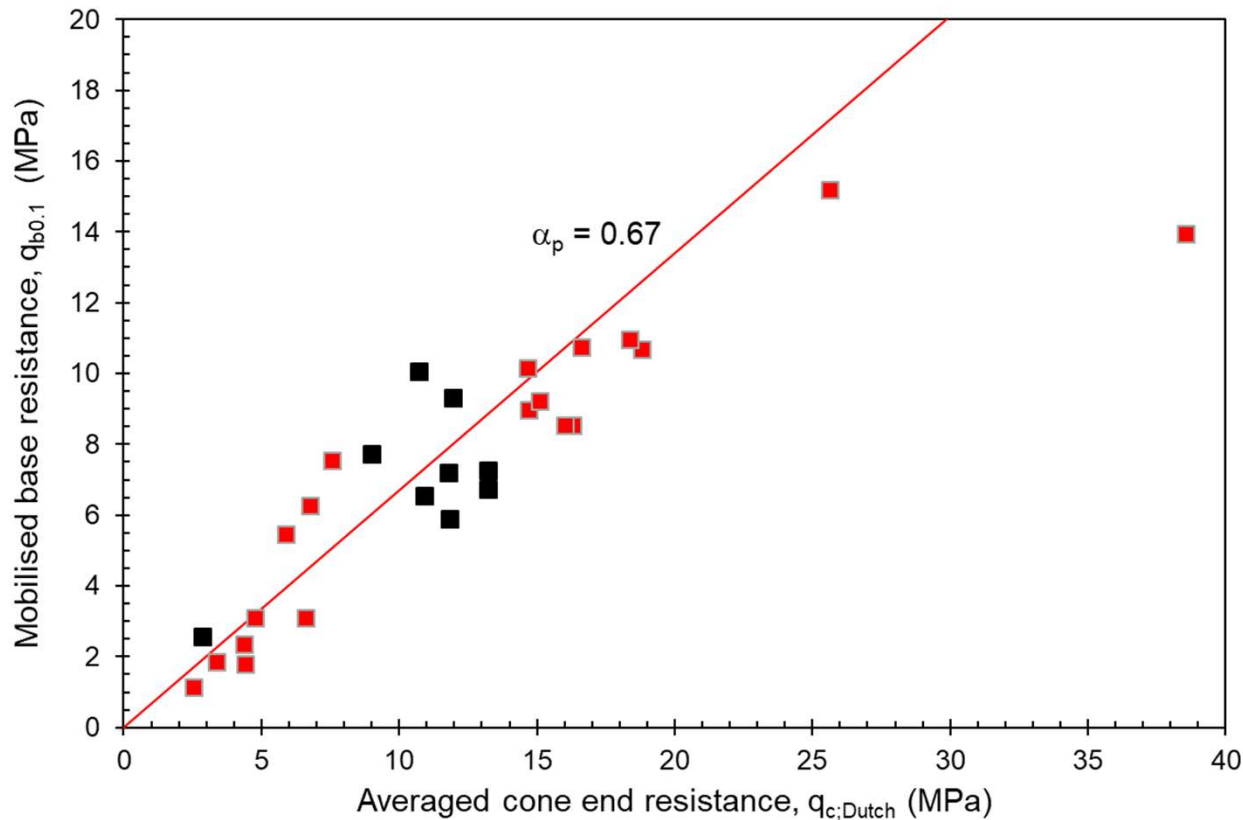
- D_{eq} ranged from 0.28 to 0.45m
- $L_{em}/D_{eq} = 1.2$ to 21
- $q_{C;Dutch} = 2.8$ to 13.2 MPa

Dutch Database of instrumented pile tests



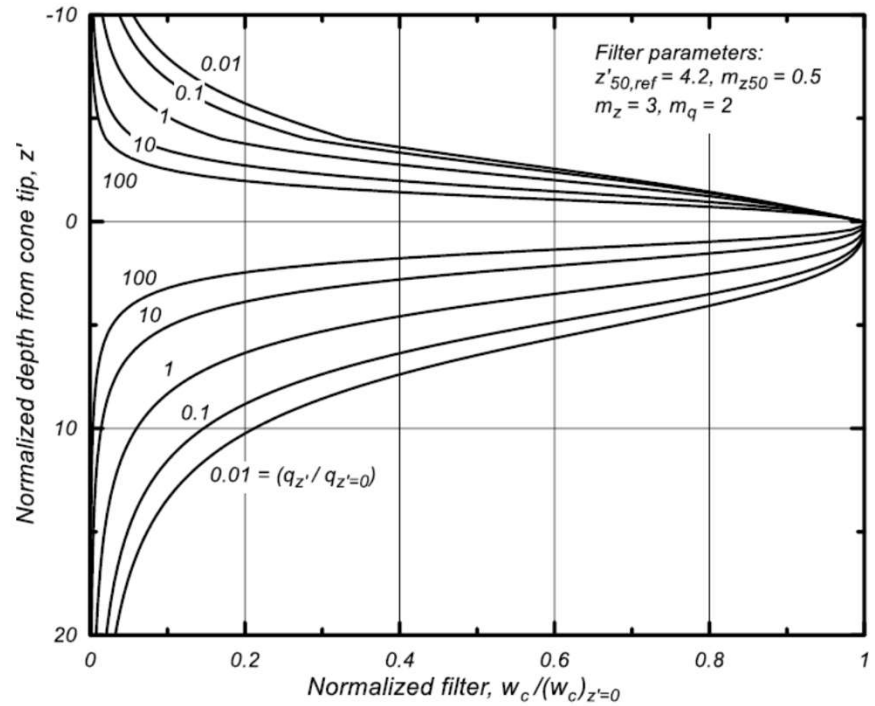
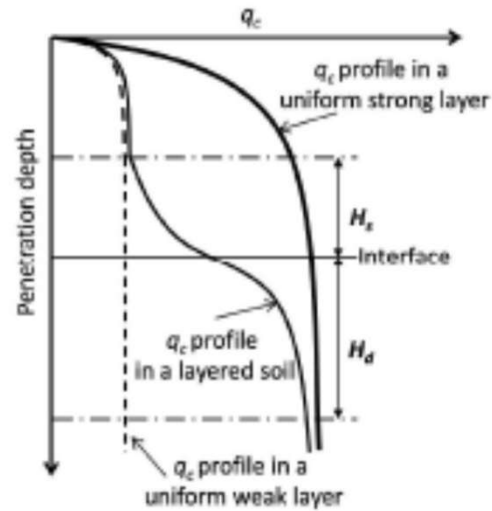
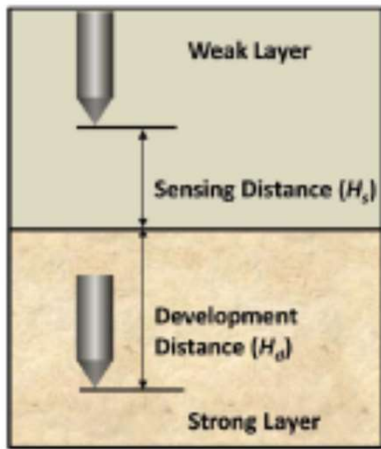
- $\alpha_p = 0.7$ gives reasonable fit
- $q_{b0.1}$ is limited to 15 Mpa which translates to constant value for $q_{c;Dutch}$ values over 21.4 MPa

Compare Dutch and ISO Databases $q_{c;Dutch}$

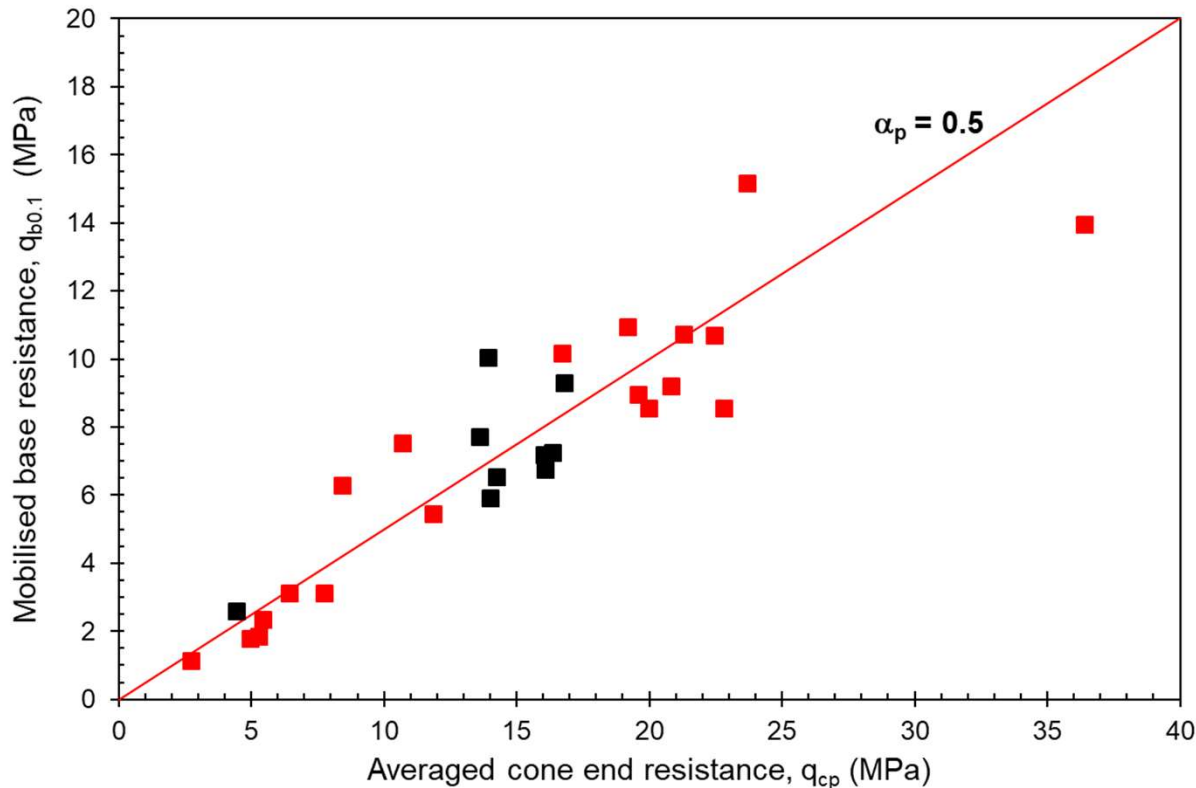


- D from 0.2 to 0.7m
- $L_{em}/D_{eq} = 11$ to 55
- $q_{c;Dutch} = 2.5$ to 38.6 MPa
- $\alpha_p = 0.67$ gives best-fit
- $q_{b0.1}$ is not limited in the ISO code (but database appears to suggest it should be!)
- Statistically better fit using a filtering technique to get q_c

Compare Dutch and ISO Database $q_{c;p}$

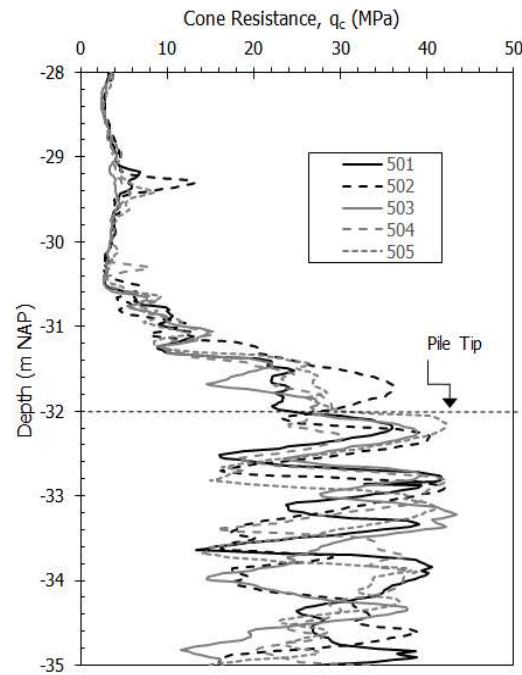
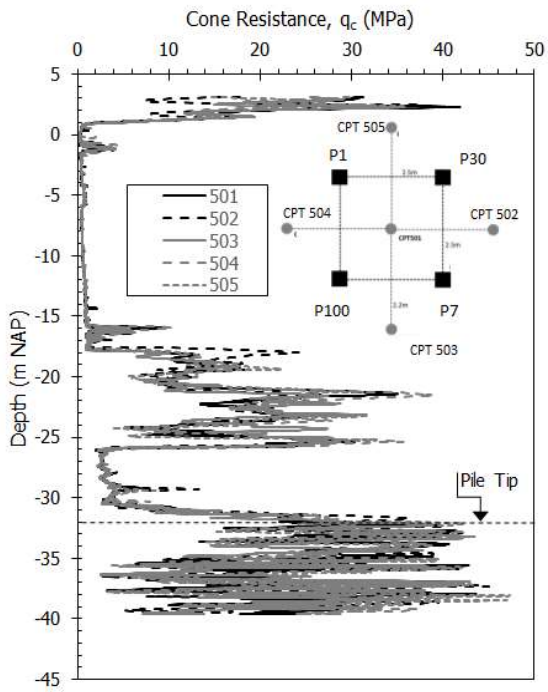


Compare Dutch and ISO Database $q_{c;p}$

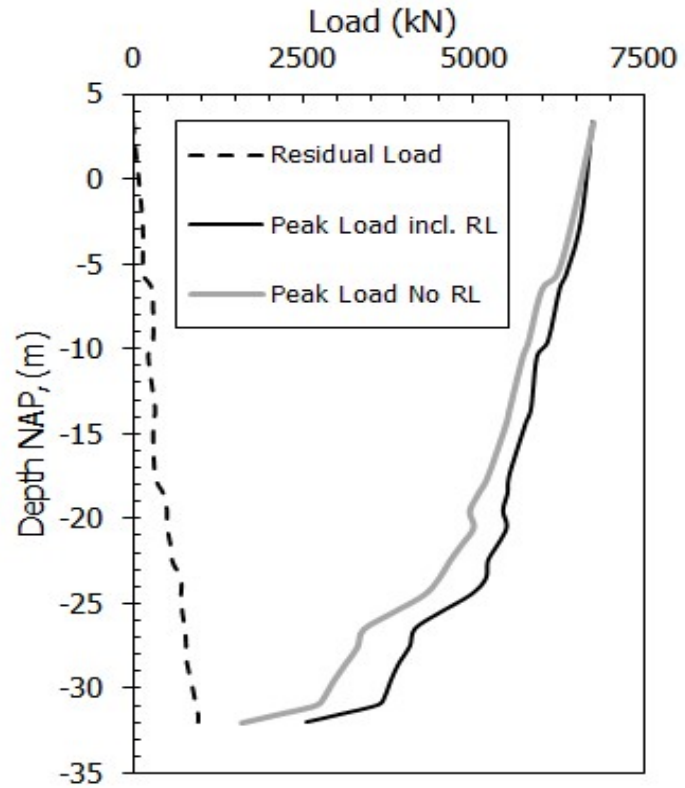
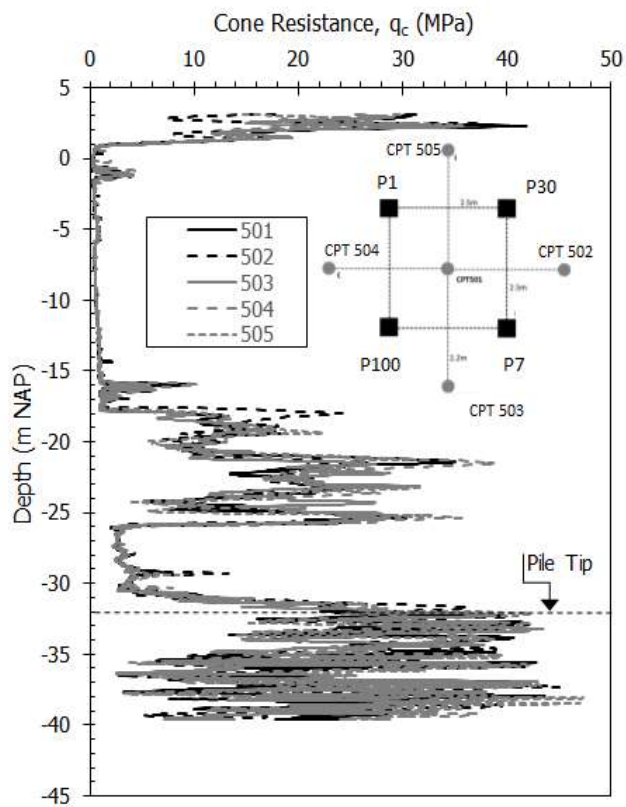


- $q_{c;p} = 2.7$ to 36.9 MPa
- $\alpha_p = 0.5$ gives best-fit
- $q_{b0.1}$ is not limited in the code (database support this)

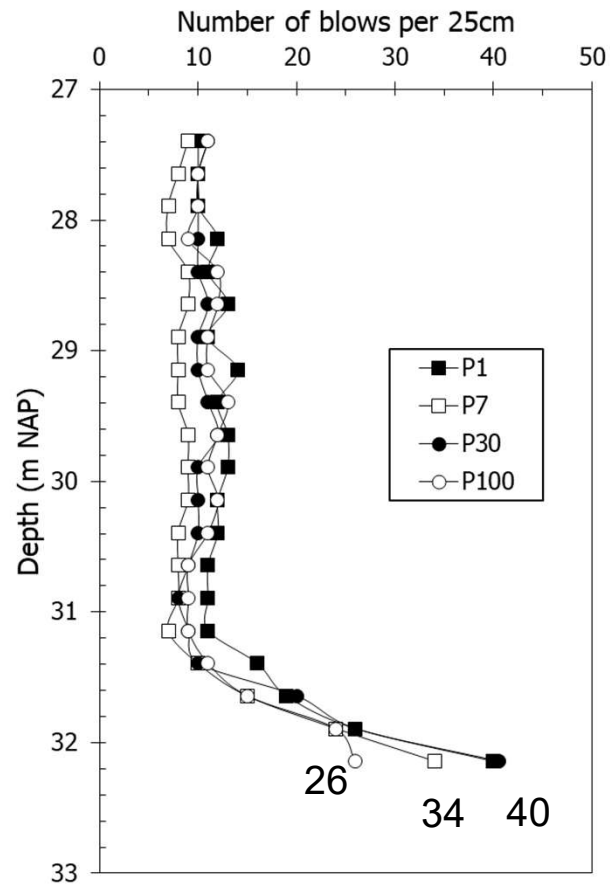
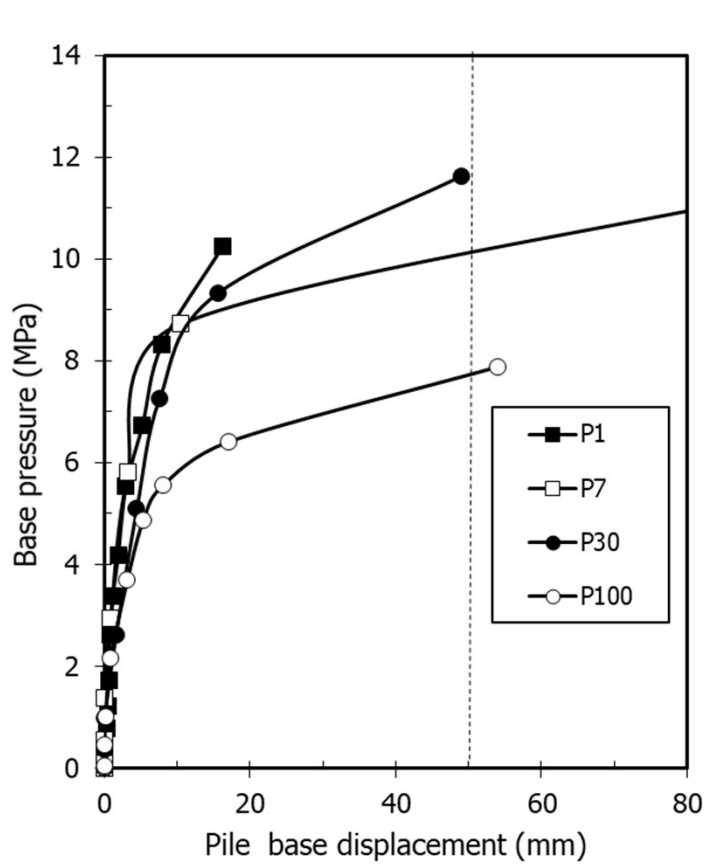
Wallhaven Pile tests 2017



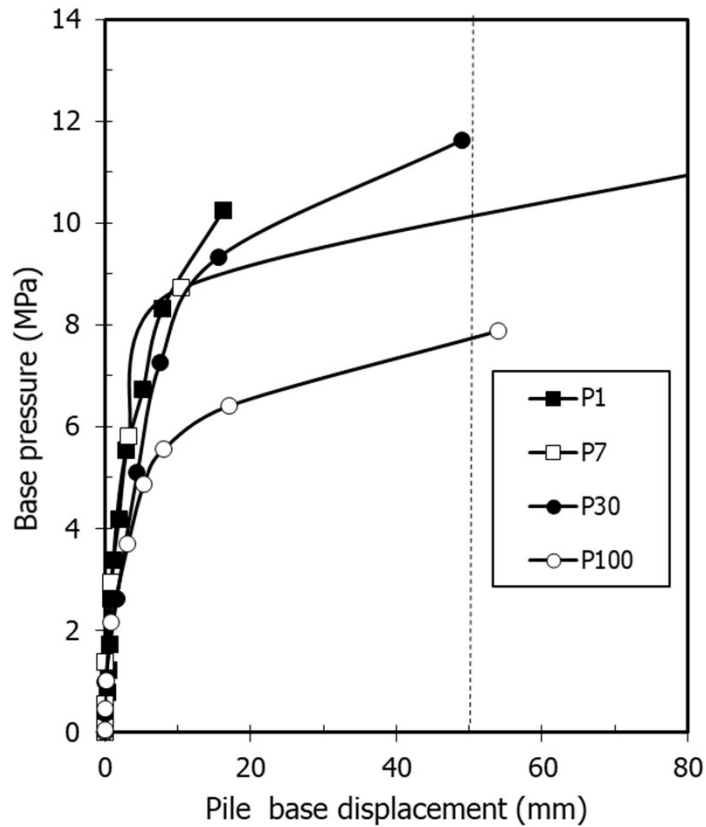
Waalhaven Pile Tests 2017



Wallhaven Pile tests 2017

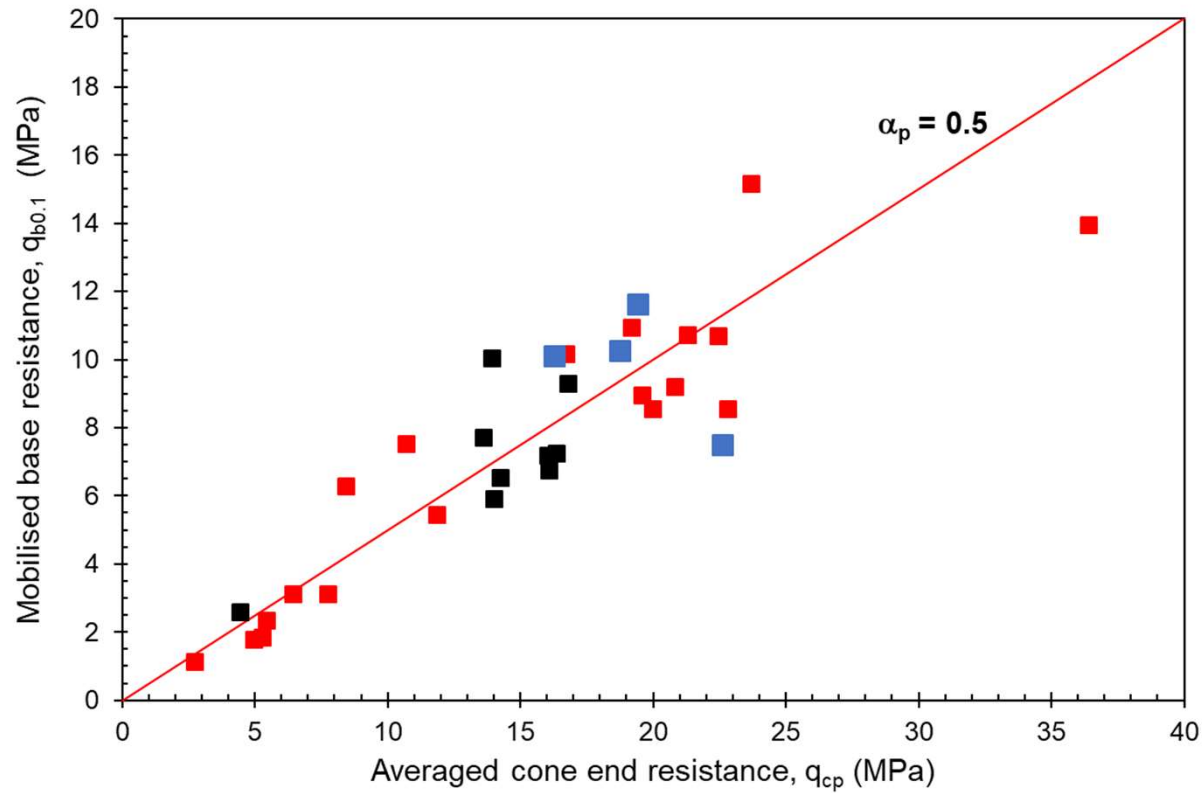


Wallhaven Pile tests 2017

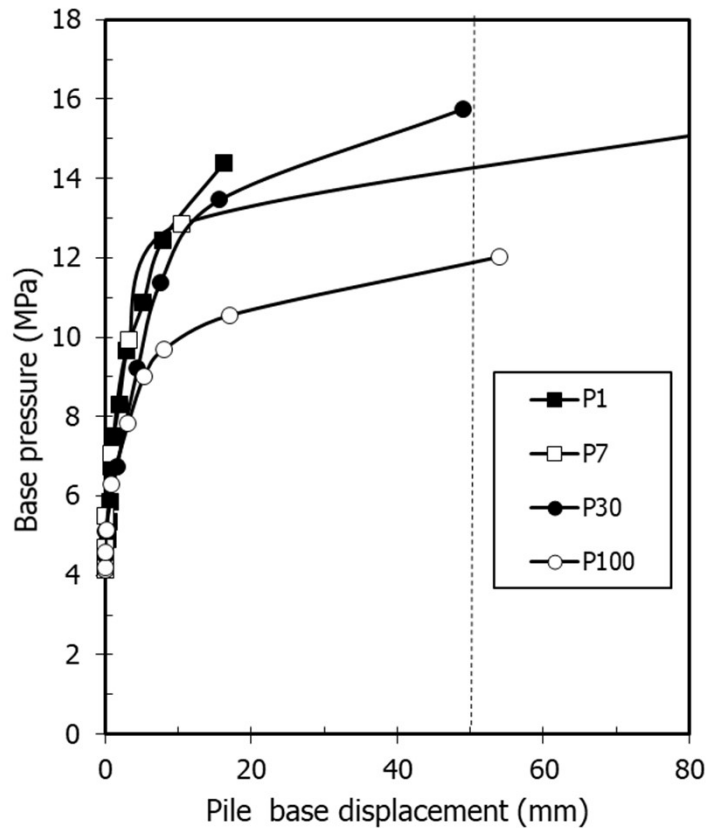


| Pile No. | q_{bL} (MPa) excluding residual | $q_{c/p}$ (MPa) | α_p |
|------------|-----------------------------------|-----------------|-------------|
| P1 | 10.26* | 18.74 | 0.55 |
| P7 | 10.12 | 16.29 | 0.62 |
| P30 | 11.62 | 19.41 | 0.60 |
| P100 | 7.49 | 22.59 | 0.33 |
| Ave | 9.87 | 19.26 | 0.52 |

Wallhaven Pile tests 2017



Wallhaven Pile tests 2017



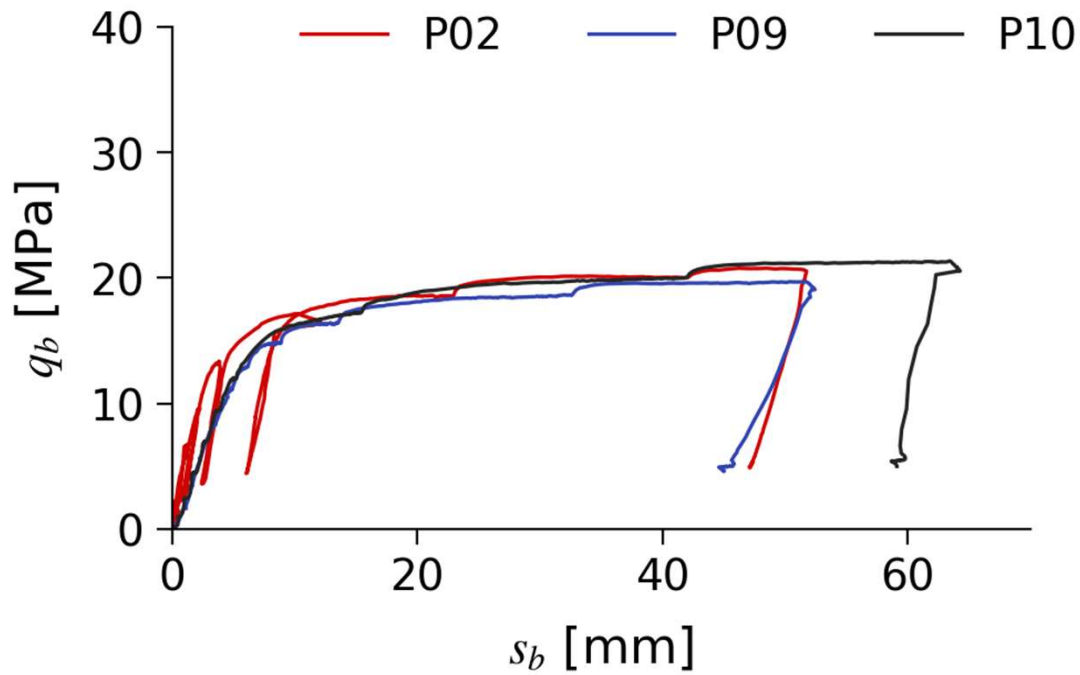
| Pile No. | q_{bL} Res (MPa) | $\alpha_{p;}$ |
|------------|--------------------|---------------|
| P1 | 14.39* | 0.77 |
| P7 | 14.25 | 0.87 |
| P30 | 15.76 | 0.81 |
| P100 | 12.01 | 0.53 |
| Ave | 14.10 | 0.75 |

Including residual load

Massvlakte II Pile Tests 2019

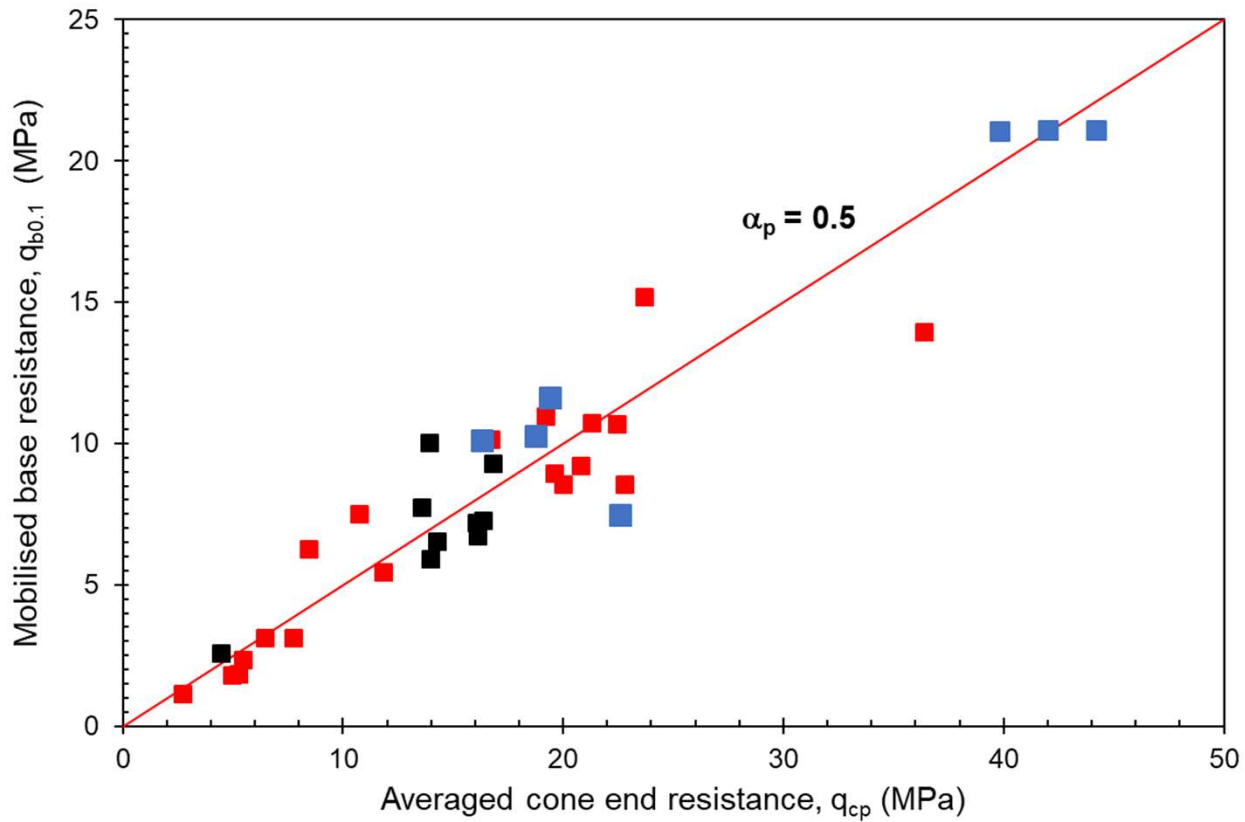


Massvlakte II Pile Tests 2019

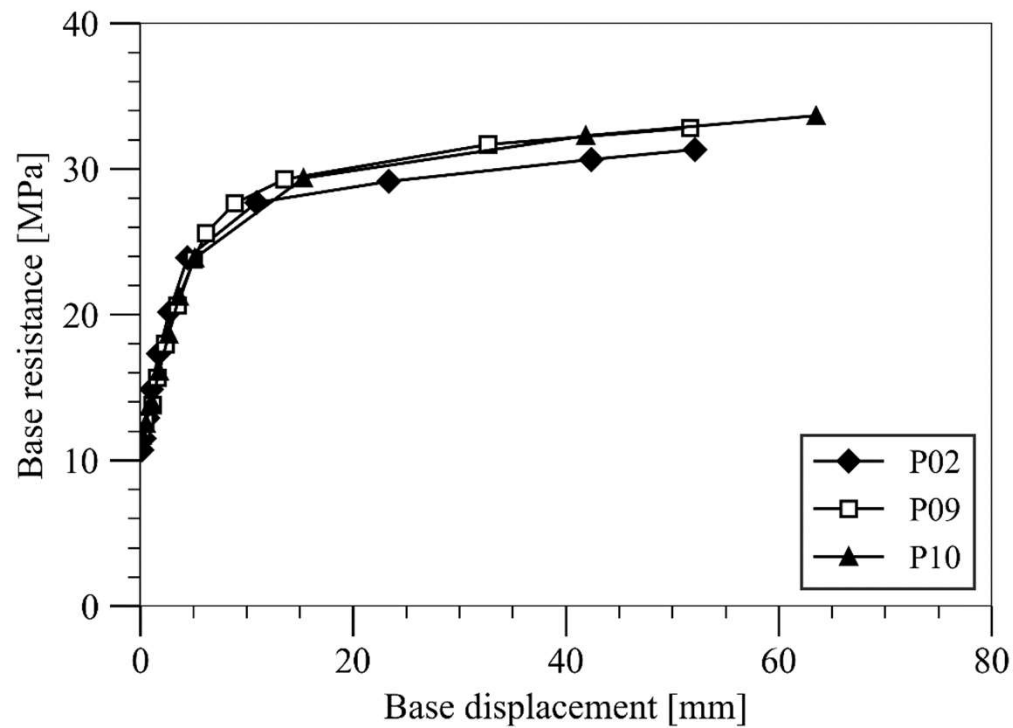


| Pile No. | $q_{b0.1}$ (MPa) excluding residual | $q_{c;p}$ (MPa) | α_p |
|------------|-------------------------------------|-----------------|-------------|
| P02 | 21.9 | 42 | 0.52 |
| P09 | 21.6 | 39.8 | 0.54 |
| P10 | 21.9 | 44.2 | 0.50 |
| Ave | | | 0.52 |

Compare Dutch and ISO Database

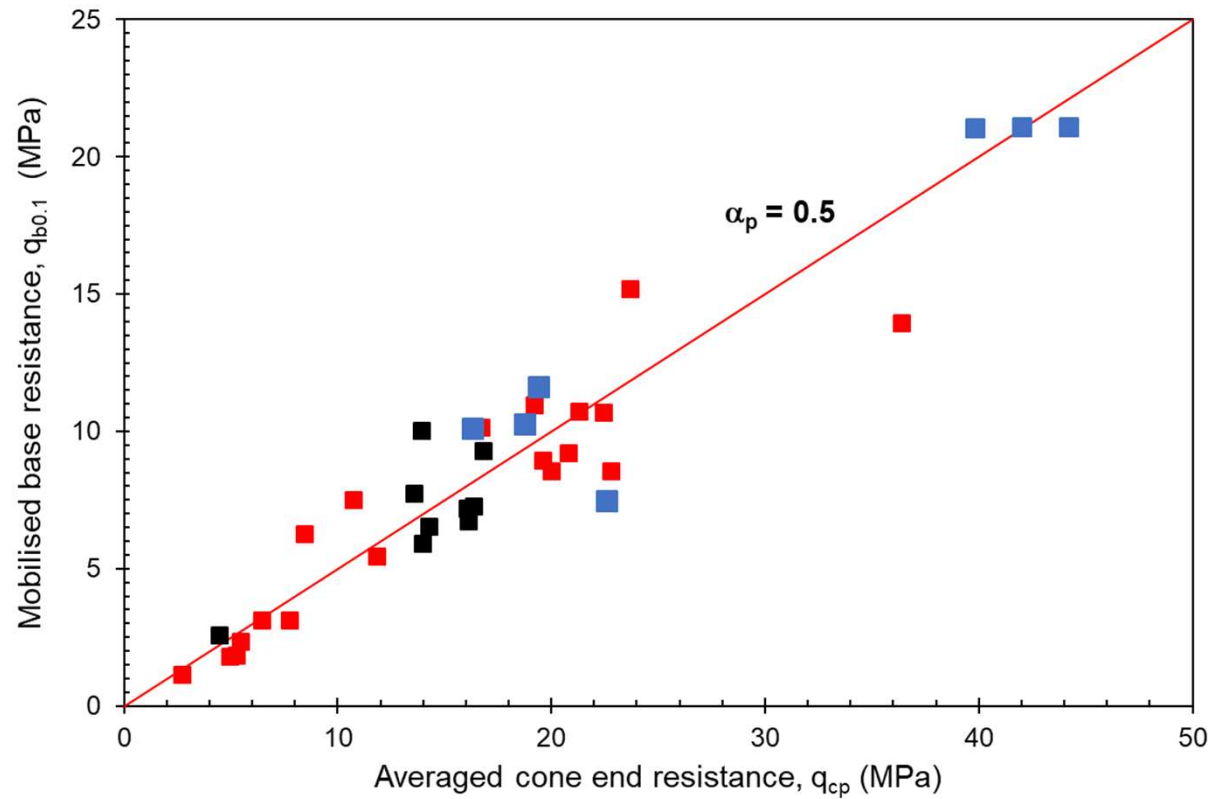


Massvlakte II Pile Tests 2019

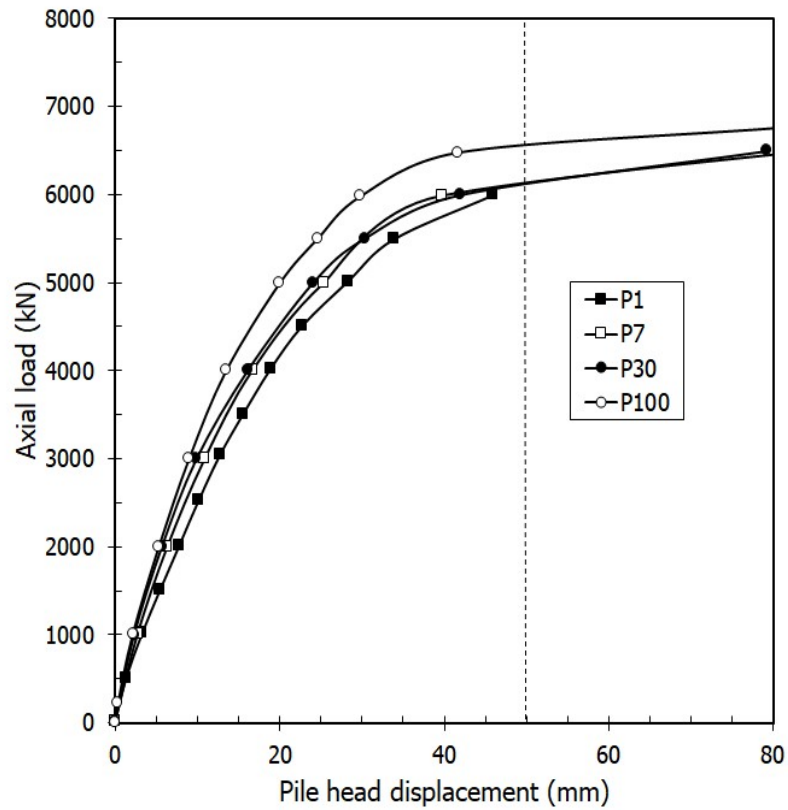


| Pile No. | $q_{b0.1}$ (MPa) excluding residual | $q_{c;p}$ (MPa) | α_p |
|------------|-------------------------------------|-----------------|-------------|
| P02 | 31.9 | 42 | 0.76 |
| P09 | 31.6 | 39.8 | 0.72 |
| P10 | 31.9 | 44.2 | 0.72 |
| Ave | | | 0.73 |

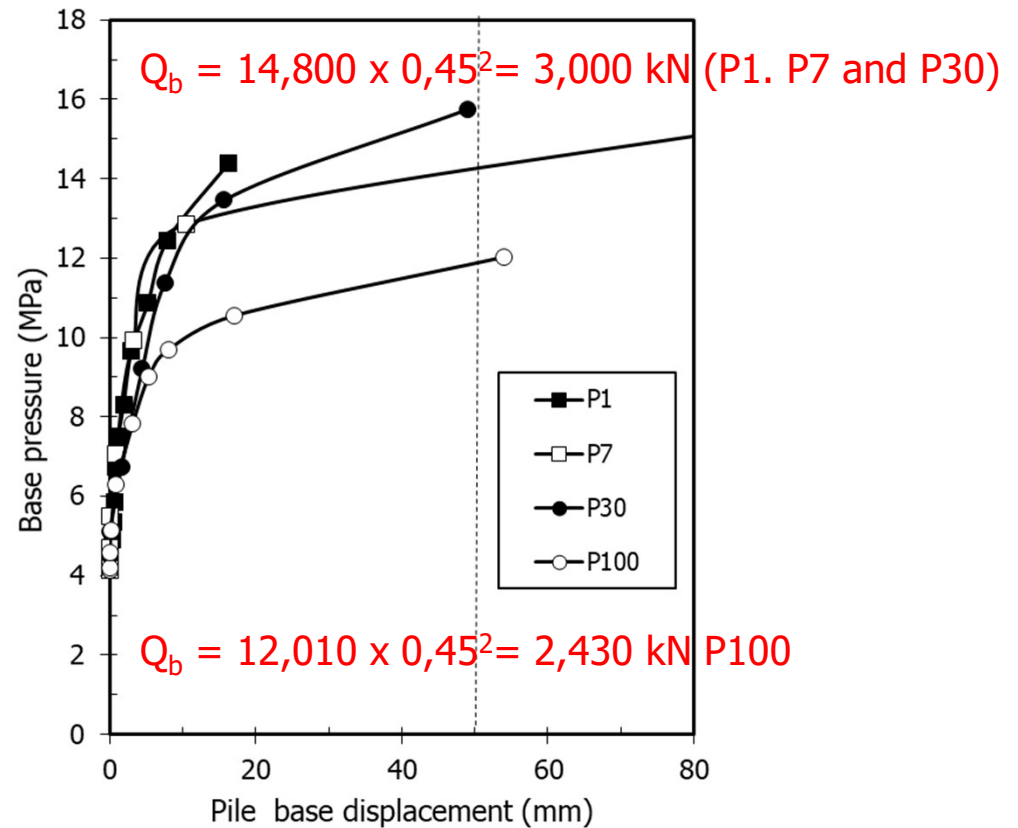
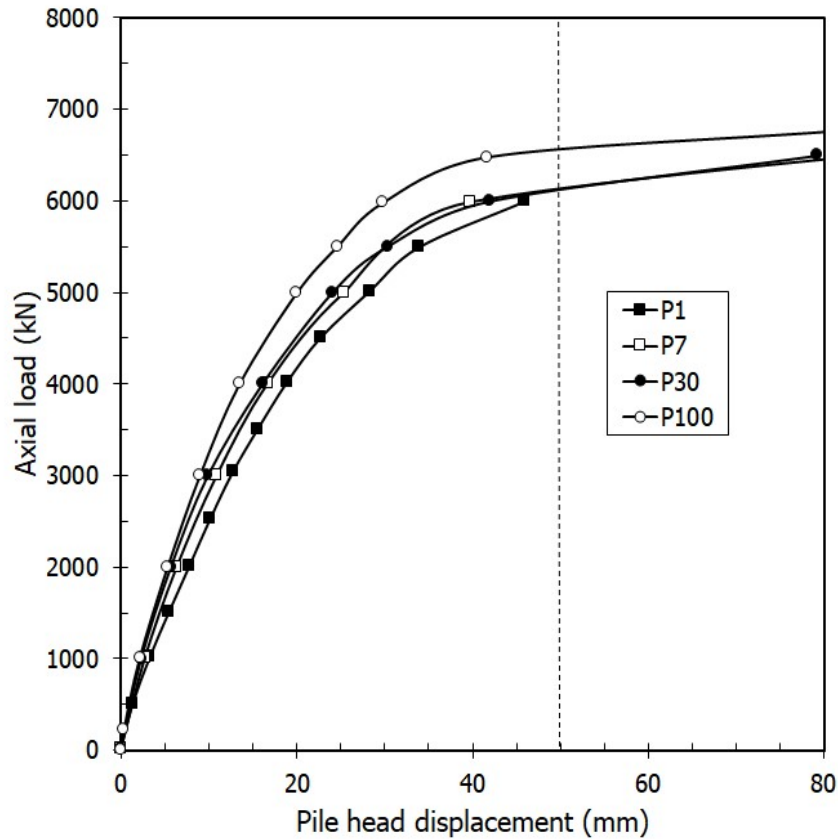
Base resistance of pre-cast concrete piles



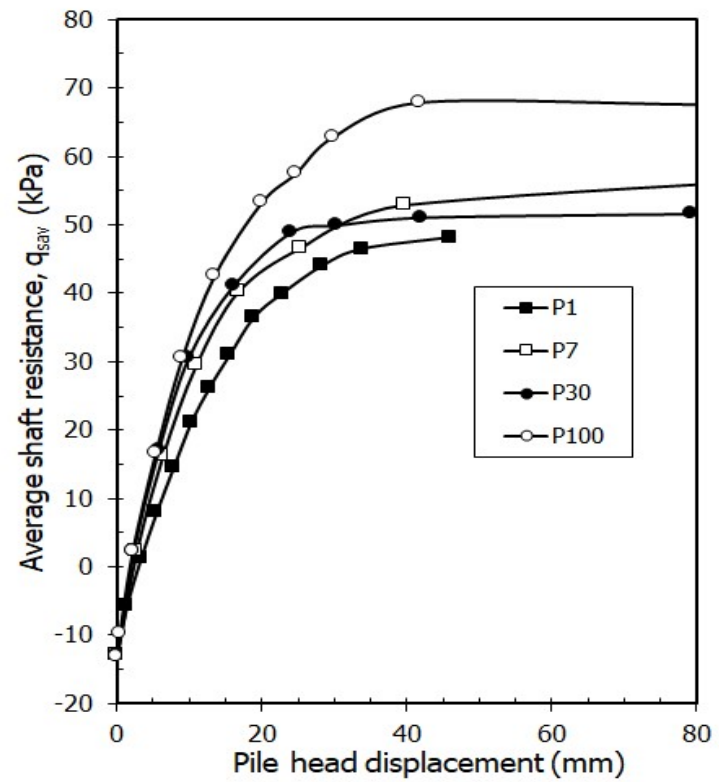
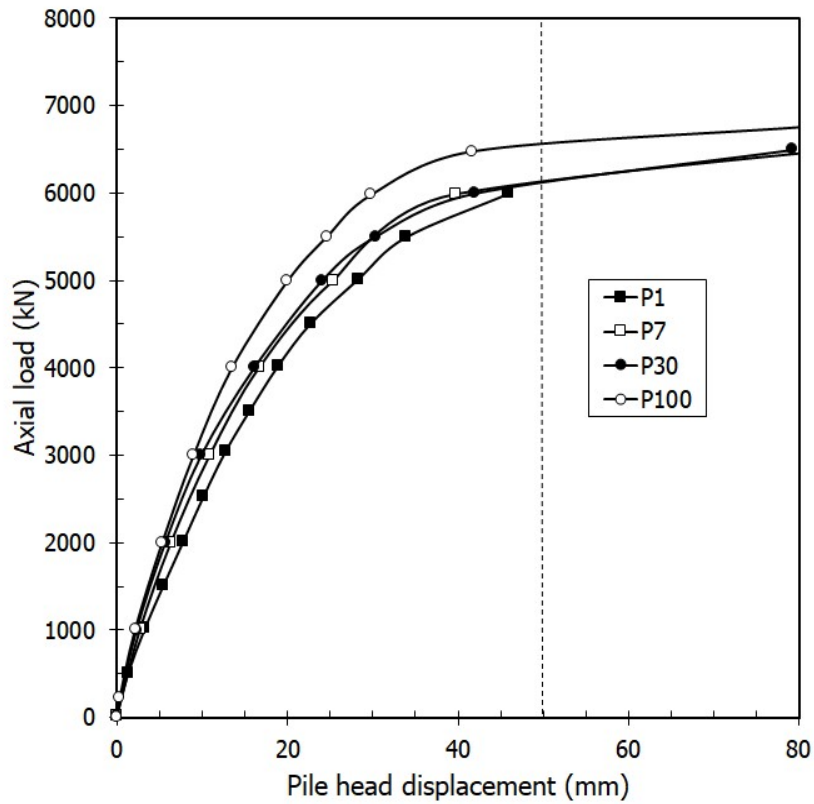
What about disappointing results?



Waalhaven Pile Tests 2017

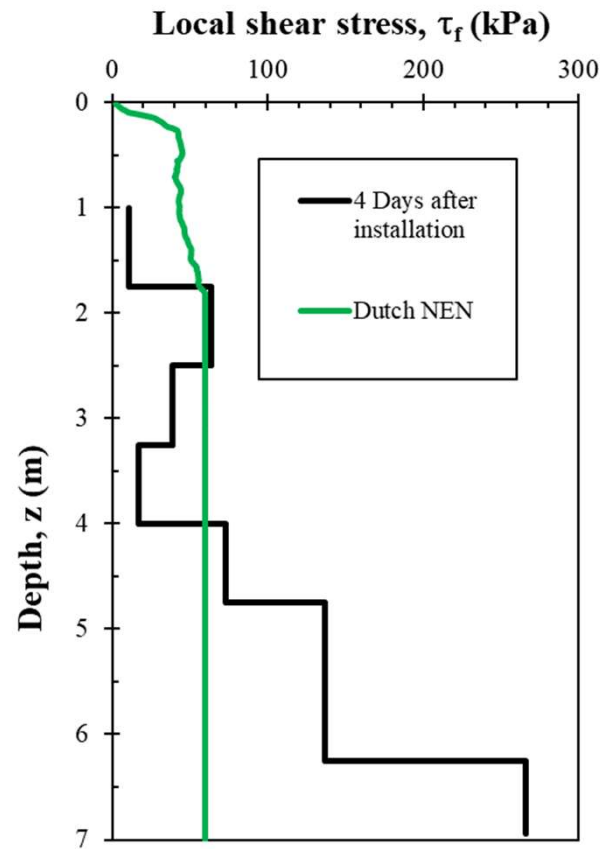
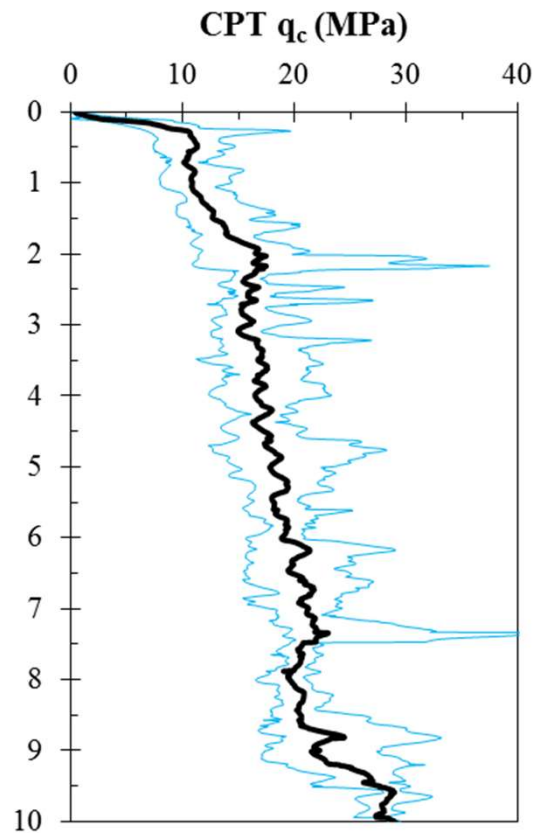


Waalhaven Pile Tests 2017

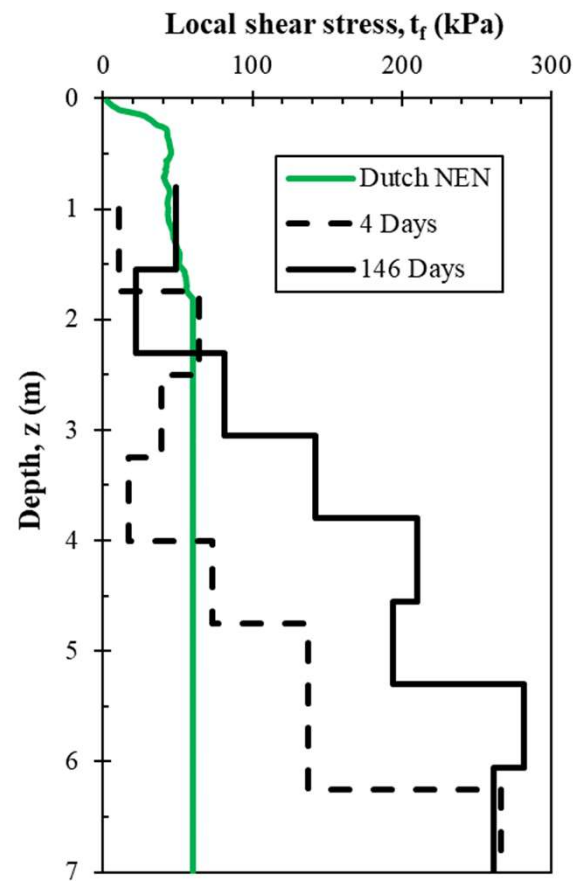
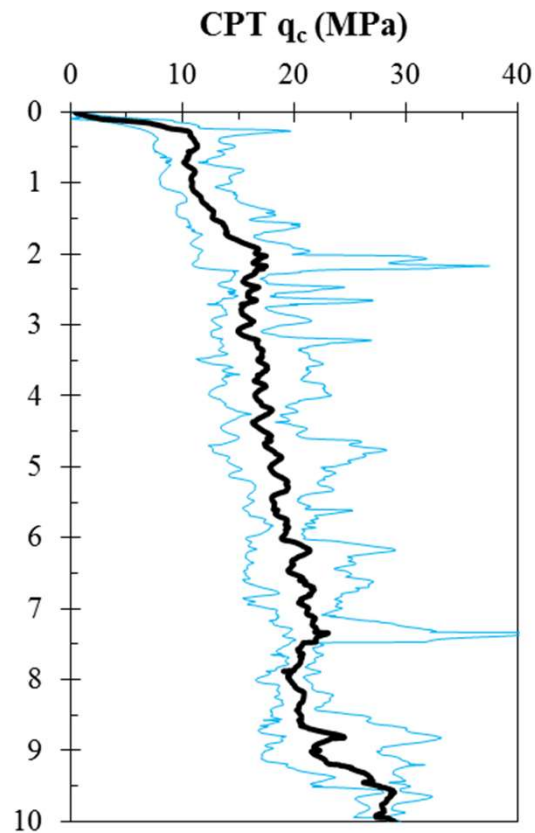




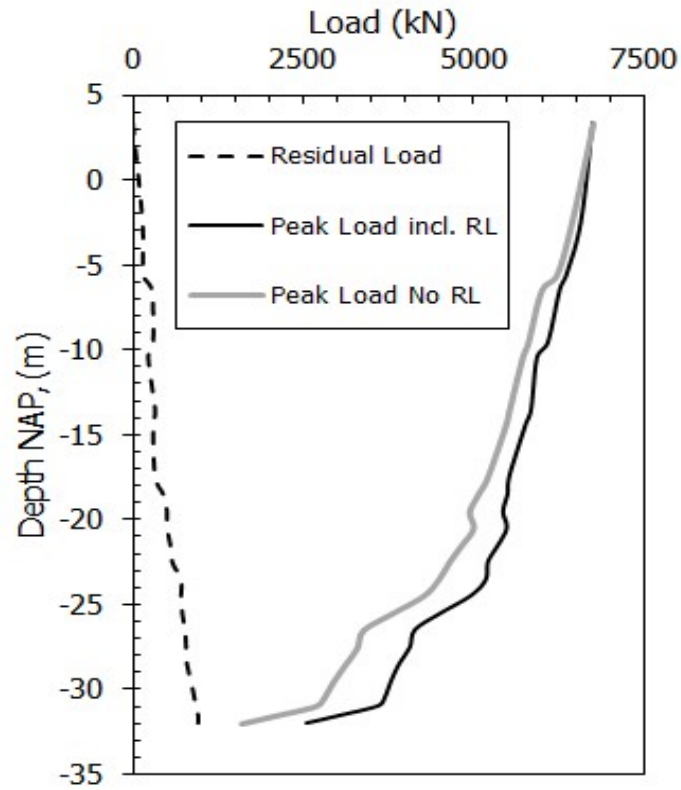
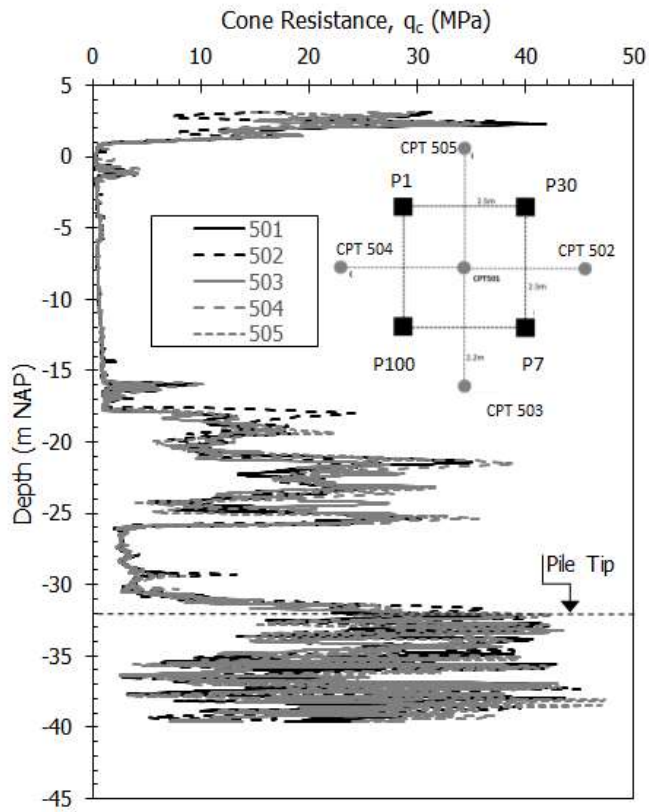
Tension Tests on Open-Tube Piles



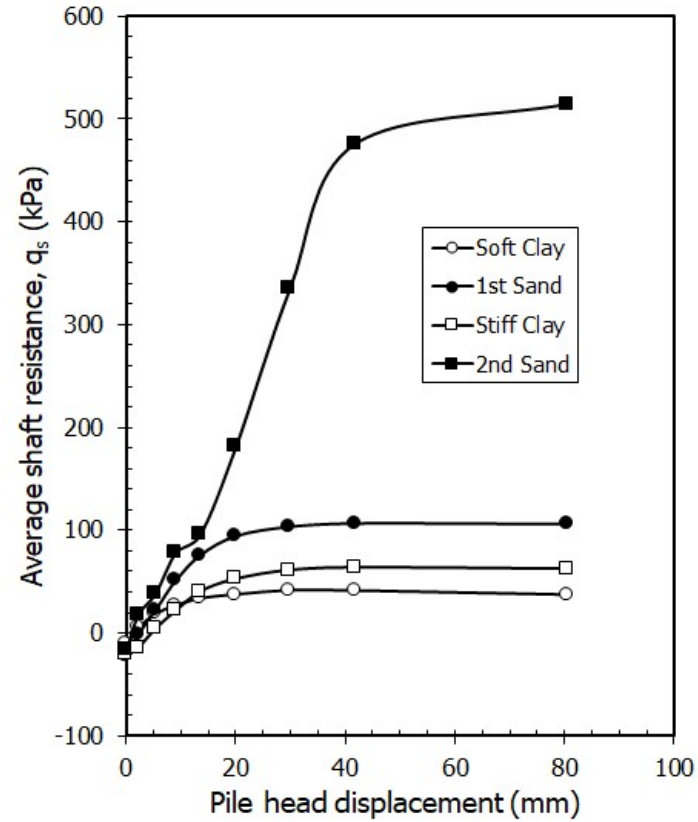
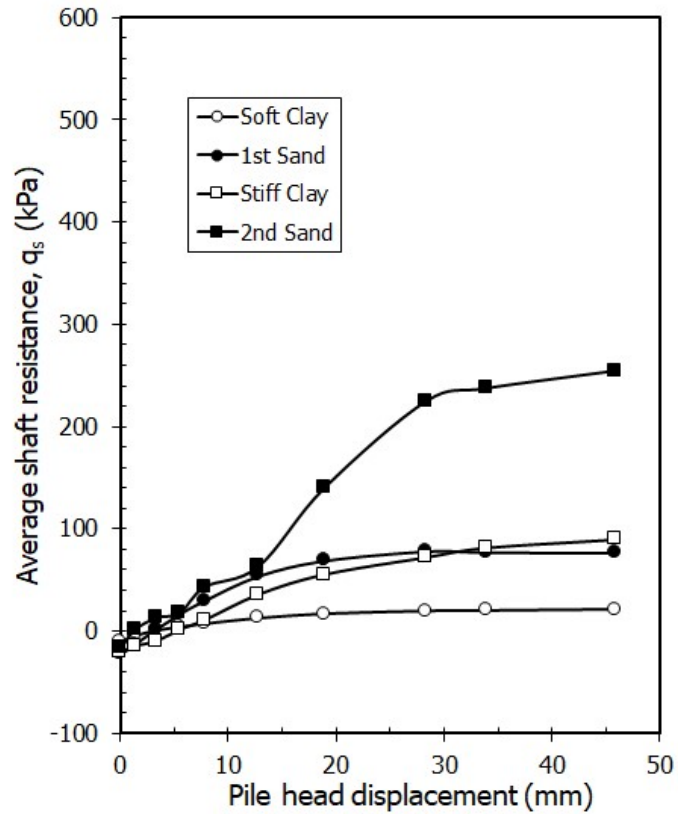
Tension Tests on Open-Tube Piles



Wallhaven Pile tests 2017



Waalhaven Pile Tests 2017



Can we use test results widely?

- PISA project Oxord/Imperial/Delft
- Load tests on 0.273, 0.762 and 2m
- Two sites, one dense sand and one stiff clay
- Method was adopted for design around the world before the end of the project
- Now used in all offshore wind projects for pile +10m



Conclusions

Base Resistance

- Results from international database of end bearing resistance in very good agreement with Dutch database
- $\alpha_p = 0.5$ with no limiting value of base resistance
- Where we measure residual loads carefully it base resistance was typically 40% higher than predicted using this approach

Instrumentation gives clear insight into mechanisms controlling pile capacity

Questions?
